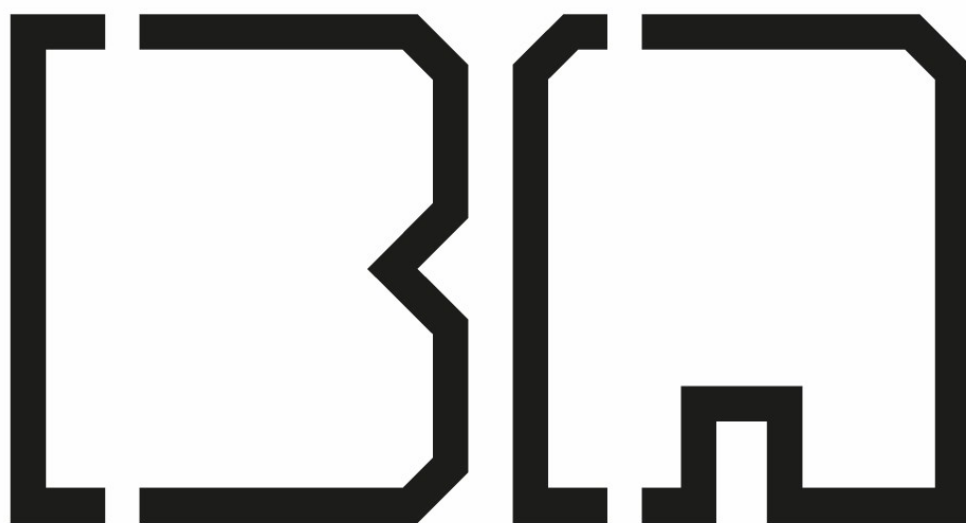


Politechnika Lubelska

Wydział Budownictwa i Architektury



Mechanika Budowli II

Projekt 2: Drgania

Wykonał:

.....

Sprawdził:

dr inż. Jakub Gontarz

PRZYKŁADOWY TEMAT

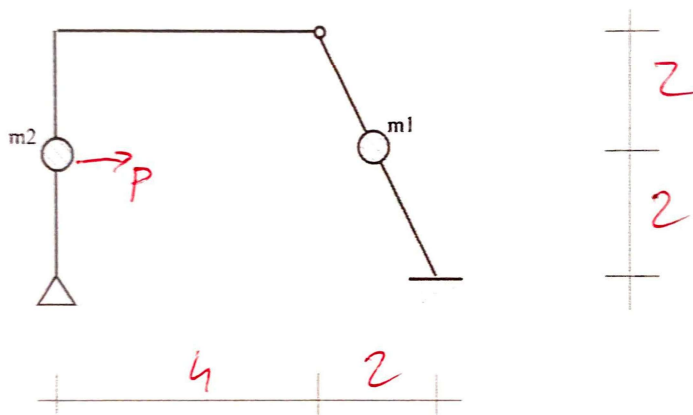
Student

Grupa

Rok akademicki

ĆWICZENIE PROJEKTOWE NR 2 Z MECHANIKI BUDOWLI

1. Wyznaczyć częstotliwości drgań własnych oraz naszkicować postacie tych drgań.
2. Wyznaczyć amplitudy sił wewnętrznych dla konstrukcji obciążonej siłą wymuszającą $P = P_0 \sin \theta t$.



$$m1 = 500 \text{ kg}$$

$$m2 = 300 \text{ kg}$$

$$m3 =$$

$$E = \text{stal}$$

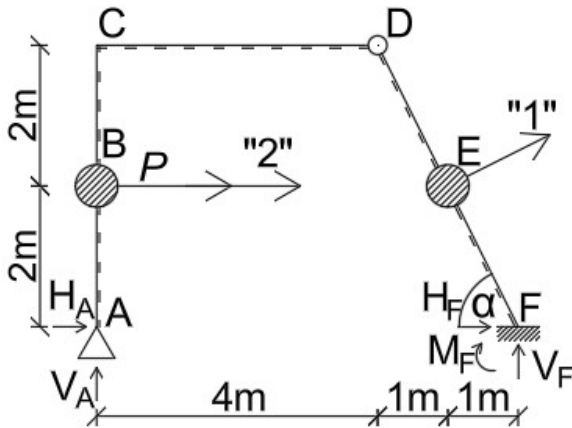
$$I = \text{IPN 16.0}$$

$$\theta = 5 \text{ Hz}$$

$$P_0 = 11 \text{ kN}$$

G

$$\text{kNm} := \text{kN} \cdot \text{m}$$



$$E := 210 \text{ GPa}$$

$$\theta := 5 \text{ Hz} \cdot 2 \pi = 31.416 \frac{\text{rad}}{\text{s}}$$

$$P_0 := 11 \text{ kN}$$

Przekrój: IPN 160

$$J := 935 \text{ cm}^4$$

$$EJ := E \cdot J = 1963.5 \text{ kN} \cdot \text{m}^2$$

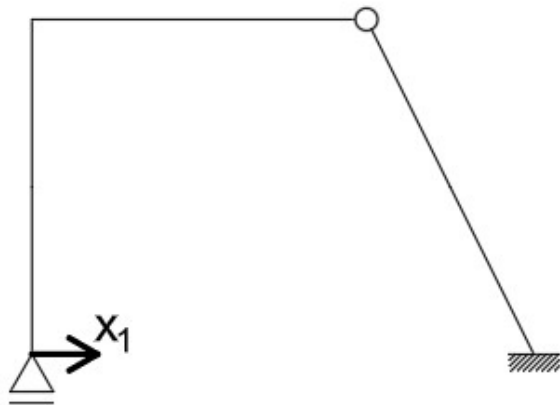
Długość pręta ukośnego:

$$L := \sqrt{(2 \text{ m})^2 + (4 \text{ m})^2} = 4.472 \text{ m}$$

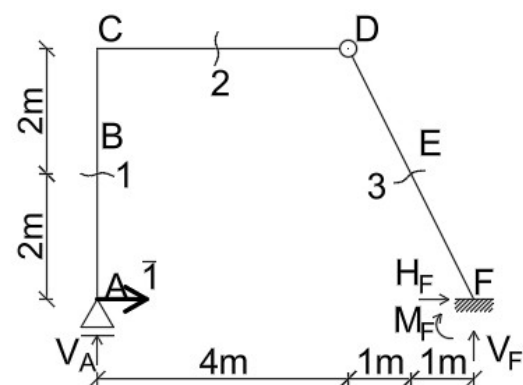
$$\sin \alpha := \frac{4 \text{ m}}{L} = 0.894 \quad \cos \alpha := \frac{2 \text{ m}}{L} = 0.447$$

Drgania własne

UPMS



Stan 1



Reakcje:

$$\Sigma M_D^L = V_A \cdot 4 \text{ m} - 1 \cdot 4 \text{ m} = 0$$

$$V_A := \frac{1 \cdot 4 \text{ m}}{4 \text{ m}} = 1$$

$$\Sigma Y = V_A + V_F = 0$$

$$V_F := -V_A = -1$$

$$\Sigma X = 1 + H_F = 0$$

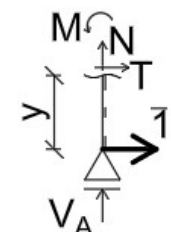
$$H_F := -1$$

$$\Sigma M_D^P = -V_F \cdot 2 \text{ m} - H_F \cdot 4 \text{ m} + M_F = 0$$

$$M_F := V_F \cdot 2 \text{ m} + H_F \cdot 4 \text{ m} = -6 \text{ m}$$

Pręt 1

$$y \in (0; 4) \text{ m}$$



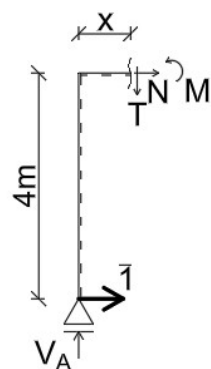
$$M_1(y) := -1 \cdot y$$

$$M_1(0 \text{ m}) = 0 \text{ m}$$

$$M_1(4 \text{ m}) = -4 \text{ m}$$

Pręt 2

$$x \in (0; 4) \text{ m}$$



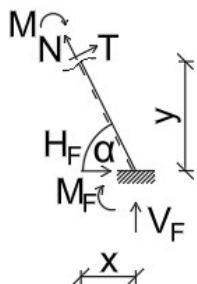
$$M_2(x) := -1 \cdot 4 \text{ m} + V_A \cdot x$$

$$M_2(0 \text{ m}) = -4 \text{ m} \quad M_2(4 \text{ m}) = 0 \text{ m}$$

Pręt 3

$$x \in (0; 2) \text{ m}$$

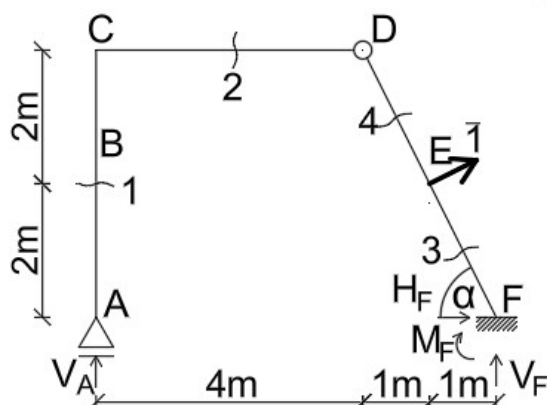
$$y(x) := 2x$$



$$M_3(x) := V_F \cdot x + H_F \cdot y(x) - M_F$$

$$M_3(0 \text{ m}) = 6 \text{ m} \quad M_3(2 \text{ m}) = 0 \text{ m}$$

Stan P1



Reakcje:

$$\Sigma M_D^L = V_A \cdot 4 \text{ m} = 0$$

$$V_A := 0$$

$$\Sigma Y = V_A + V_F + 1 \cdot \cos \alpha = 0$$

$$V_F := -V_A - 1 \cdot \cos \alpha = -0.447$$

$$\Sigma X = H_F + 1 \cdot \sin \alpha = 0$$

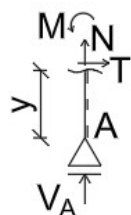
$$H_F := -1 \cdot \sin \alpha = -0.894$$

$$\Sigma M_D^P = -1 \cdot \frac{L}{2} - V_F \cdot 2 \text{ m} - H_F \cdot 4 \text{ m} + M_F = 0$$

$$M_F := 1 \cdot \frac{L}{2} + V_F \cdot 2 \text{ m} + H_F \cdot 4 \text{ m} = -2.236 \text{ m}$$

Pręt 1

$$y \in (0; 4) \text{ m}$$

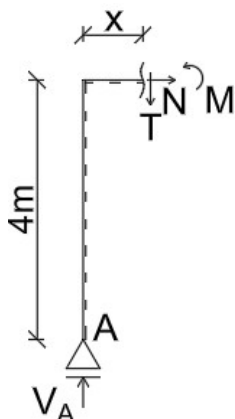


$$M_1(y) := 0$$

$$M_1(0 \text{ m}) = 0 \text{ m} \quad M_1(4 \text{ m}) = 0$$

Pręt 2

$$x \in (0; 4) \text{ m}$$



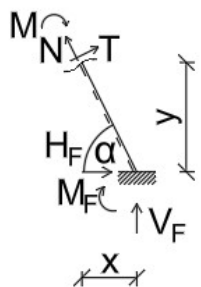
$$M_2(x) := -1 \cdot 4 \text{ m} + V_A \cdot x$$

$$M_2(0 \text{ m}) = -4 \text{ m} \quad M_2(4 \text{ m}) = -4 \text{ m}$$

Pręt 3

$$x \in (0; 2) \text{ m}$$

$$y(x) := 2x$$



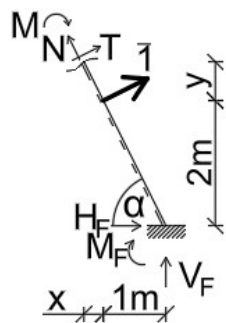
$$M_3(x) := V_F \cdot x + H_F \cdot y(x) - M_F$$

$$M_3(0 \text{ m}) = 2.236 \text{ m} \quad M_3(2 \text{ m}) = -2.236 \text{ m}$$

Pręt 4

$$x \in (0; 2) \text{ m}$$

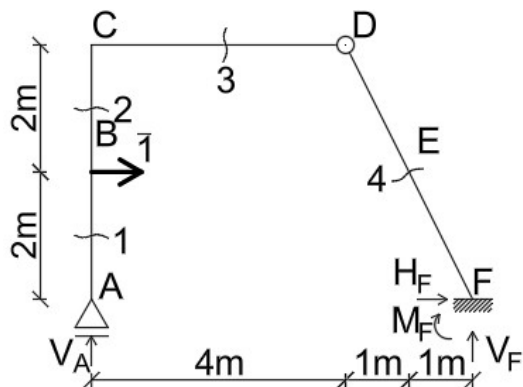
$$y(x) := 2x$$



$$M_3(x) := V_F \cdot (x + 1 \text{ m}) + H_F \cdot (y(x) + 2 \text{ m}) + 1 \cdot \sqrt{x^2 + y(x)^2} - M_F$$

$$M_3(0 \text{ m}) = 0 \text{ m} \quad M_3(2 \text{ m}) = 0 \text{ m}$$

Stan P2



Reakcje:

$$\Sigma M_D^L = V_A \cdot 4 \text{ m} - 1 \cdot 2 \text{ m} = 0$$

$$V_A := \frac{1 \cdot 2 \text{ m}}{4 \text{ m}} = 0.5$$

$$\Sigma Y = V_A + V_F = 0$$

$$V_F := -V_A = -0.5$$

$$\Sigma X = 1 + H_F = 0$$

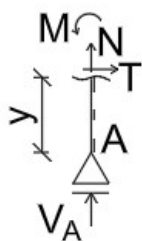
$$H_F := -1$$

$$\Sigma M_D^P = -V_F \cdot 2 \text{ m} - H_F \cdot 4 \text{ m} + M_F = 0$$

$$M_F := V_F \cdot 2 \text{ m} + H_F \cdot 4 \text{ m} = -5 \text{ m}$$

Pręt 1

$$y \in (0; 2) \text{ m}$$



$$M_1(y) := 0$$

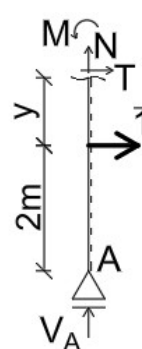
$$M_1(0 \text{ m}) = 0 \text{ m} \quad M_1(2 \text{ m}) = 0$$

Pręt 2

$$y \in (0; 2) \text{ m}$$

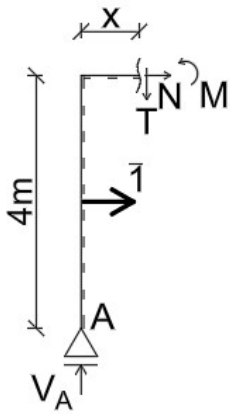
$$M_2(y) := -1 \cdot y$$

$$M_2(0 \text{ m}) = 0 \text{ m} \quad M_2(2 \text{ m}) = -2 \text{ m}$$



Pręt 3

$$x \in (0; 4) \text{ m}$$



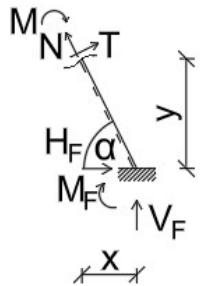
$$M_3(x) := -1 \cdot 2 \text{ m} + V_A \cdot x$$

$$M_3(0 \text{ m}) = -2 \text{ m} \quad M_3(4 \text{ m}) = 0 \text{ m}$$

Pręt 4

$$x \in (0; 2) \text{ m}$$

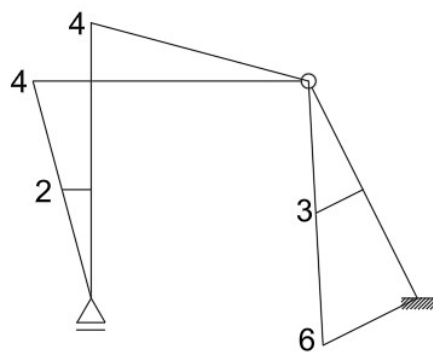
$$y(x) := 2x$$



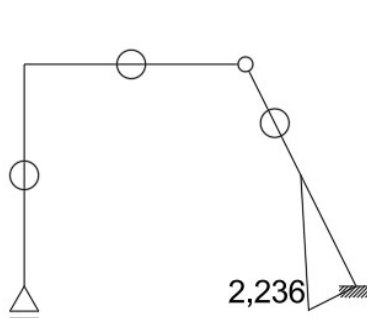
$$M_4(x) := V_F \cdot x + H_F \cdot y(x) - M_F$$

$$M_4(0 \text{ m}) = 5 \text{ m} \quad M_4(2 \text{ m}) = 0 \text{ m}$$

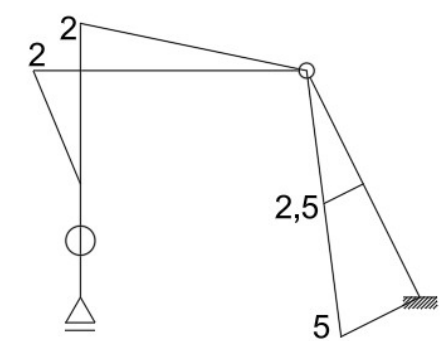
M1 [m]



MP1 [m]



MP2 [m]



$$M_1 := \begin{bmatrix} 0 & -2 \\ -2 & -4 \\ -4 & 0 \\ 0 & 3 \\ 3 & 6 \end{bmatrix} \text{ m}$$

$$M_{P1} := \begin{bmatrix} 0 & 0 \\ 0 & 0 \\ 0 & 0 \\ 0 & 0 \\ 0 & 2.236 \end{bmatrix} \text{ m}$$

$$M_{P2} := \begin{bmatrix} 0 & 0 \\ 0 & -2 \\ -2 & 0 \\ 0 & 2.5 \\ 2.5 & 5 \end{bmatrix} \text{ m}$$

$$\delta_{11} = \frac{1}{EJ} \left[\begin{array}{c} 4 \quad 4 \\ \diagdown \quad \diagup \\ 4 \quad 4 \end{array} + \begin{array}{c} 4 \\ \diagdown \quad \diagup \\ 4 \end{array} + \begin{array}{c} 6 \quad 6 \\ \diagdown \quad \diagup \\ 6 \end{array} \right]$$

$$\delta_{11} := \frac{\text{m}^3}{EJ} \left(\frac{1}{2} \cdot 4 \cdot 4 \cdot \frac{2}{3} \cdot 4 + \frac{1}{2} \cdot 4 \cdot 4 \cdot \frac{2}{3} \cdot 4 + \frac{1}{2} \cdot 6 \cdot 4.472 \cdot \frac{2}{3} \cdot 6 \right) = 96.331 \frac{\text{m}^3}{EJ}$$

$$\delta_{11} = 0.04906 \frac{\text{m}}{\text{kN}}$$

$$\delta_{1P1} = \frac{1}{EJ} \left[\begin{array}{c} 3 \\ \diagdown \quad \diagup \\ 6 \end{array} \quad \begin{array}{c} 1/2 \\ \diagdown \quad \diagup \\ 2.236 \end{array} \right]$$

$$\delta_{1P1} := \frac{\text{m}^3}{EJ} \left(\frac{1}{2} \cdot 2.236 \cdot 2.236 \cdot \left(\frac{2}{3} \cdot 6 + \frac{1}{3} \cdot 3 \right) \right) = 12.499 \frac{\text{m}^3}{EJ}$$

$$\delta_{1P1} = 0.00637 \frac{\text{m}}{\text{kN}}$$

$$\delta_{1P2} = \frac{1}{EJ} \left[\frac{4}{2} \cdot \frac{2}{2} + \frac{4}{2} \cdot \frac{4}{2} + \frac{6}{6} \cdot \frac{5}{5} \right]$$

$$\delta_{1P2} := \frac{\text{m}^3}{EJ} \left(\frac{1}{2} \cdot 2 \cdot 2 \cdot \left(\frac{2}{3} \cdot 4 + \frac{1}{3} \cdot 2 \right) + \frac{1}{2} \cdot 4 \cdot 4 \cdot \frac{2}{3} \cdot 2 + \frac{1}{2} \cdot 6 \cdot 4 \cdot 472 \cdot \frac{2}{3} \cdot 5 \right) = 62.053 \frac{\text{m}^3}{EJ} \quad \delta_{1P2} = 0.0316 \frac{\text{m}}{\text{kN}}$$

Obliczenia niewiadomych metody sił

$$\delta_{11} \cdot x_{11} + \delta_{1P1} = 0$$

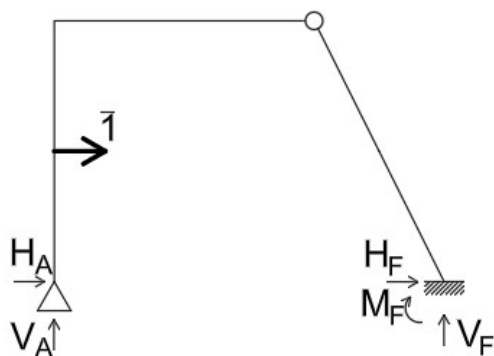
$$x_{11} := \frac{-\delta_{1P1}}{\delta_{11}} = -0.13$$

$$\delta_{11} \cdot x_{11} + \delta_{1P2} = 0$$

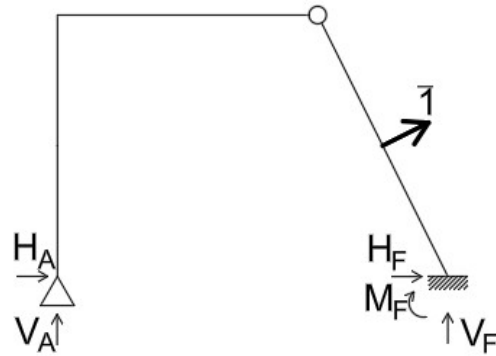
$$x_{12} := \frac{-\delta_{1P2}}{\delta_{11}} = -0.644$$

Stany ostateczne

Stan ostateczny 1



Stan ostateczny 2

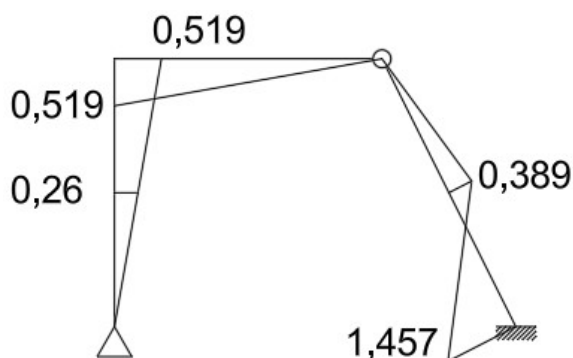


Wykresy ostateczne

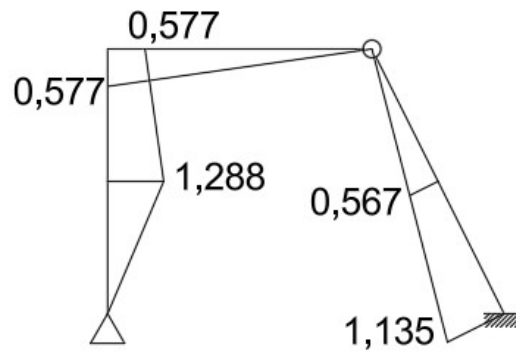
$$M_{ost1} := M_1 \cdot x_{11} + M_{P1} = \begin{bmatrix} 0 & 0.26 \\ 0.26 & 0.519 \\ 0.519 & 0 \\ 0 & -0.389 \\ -0.389 & 1.457 \end{bmatrix} \text{ m}$$

$$M_{ost2} := M_1 \cdot x_{12} + M_{P2} = \begin{bmatrix} 0 & 1.288 \\ 1.288 & 0.577 \\ 0.577 & 0 \\ 0 & 0.567 \\ 0.567 & 1.135 \end{bmatrix} \text{ m}$$

Most1 [m]



Most2 [m]



Obliczenie przemieszczeń na kierunkach swobody dynamicznej:

$$\delta_{11} = \int_S \frac{M_{ost1} \cdot M_{P1}}{EJ} dS = \frac{1}{EJ} \left[\frac{0.389}{1.457} \cdot \frac{L}{2} \right]$$

$$\delta_{11} := \frac{\text{m}^3}{EJ} \left(\frac{1}{2} \cdot 2.236 \cdot 2.236 \cdot \left(\frac{2}{3} \cdot 1.457 - \frac{1}{3} \cdot 0.389 \right) \right) = 2.104 \frac{\text{m}^3}{EJ} \quad \delta_{11} = 0.00107 \frac{\text{m}}{\text{kN}}$$

$$\delta_{12} = \int_S \frac{M_{ost2} \cdot M_{P1}}{EJ} dS = \frac{1}{EJ} \left[\begin{array}{c} 0,567 \\ \text{trapezoid} \\ 1,135 \quad 2,236 \end{array} \right]$$

$$\delta_{12} := \frac{\mathbf{m}^3}{EJ} \left(\frac{1}{2} \cdot 2.236 \cdot 2.236 \cdot \left(\frac{2}{3} \cdot 1.135 + \frac{1}{3} \cdot 0.567 \right) \right) = 2.364 \frac{\mathbf{m}^3}{EJ} \quad \delta_{12} = 0.0012 \frac{\mathbf{m}}{\mathbf{kN}} \quad \delta_{21} := \delta_{12}$$

$$\delta_{22} = \int_S \frac{M_{ost2} \cdot M_{P2}}{EJ} dS = \frac{1}{EJ} \left[\begin{array}{c} 0,577 \quad 2 \\ \text{trapezoid} \\ 1,288 \end{array} + \begin{array}{c} 2 \\ \text{triangle} \\ 0,577 \quad 4 \end{array} + \begin{array}{c} \text{triangle} \\ 1,135 \quad 5 \end{array} \right]$$

$$\delta_{22} := \frac{\mathbf{m}^3}{EJ} \left(\frac{-1}{2} \cdot 2 \cdot 2 \cdot \left(\frac{2}{3} \cdot 0.577 + \frac{1}{3} \cdot 1.288 \right) - \frac{1}{2} \cdot 2 \cdot 4 \cdot \frac{2}{3} \cdot 0.577 + \frac{1}{2} \cdot 1.135 \cdot 4.472 \cdot \frac{2}{3} \cdot 5 \right) = 5.293 \frac{\mathbf{m}^3}{EJ}$$

$$\delta_{22} = 0.0027 \frac{\mathbf{m}}{\mathbf{kN}}$$

Układ równań opisujący ruch:

$$\left(m_1 \cdot \delta_{11} - \frac{1}{\omega^2} \right) \cdot A_1 + m_2 \cdot \delta_{12} \cdot A_2 = 0$$

$$m_1 \cdot \delta_{21} \cdot A_1 + \left(m_2 \cdot \delta_{22} - \frac{1}{\omega^2} \right) \cdot A_2 = 0$$

Podstawienie: $X = \frac{m_0 \cdot \omega^2}{EJ} \mathbf{m}^3$

$$\delta'_{11} := \frac{\delta_{11} \cdot EJ}{\mathbf{m}^3} = 2.104 \quad \delta'_{12} := \frac{\delta_{12} \cdot EJ}{\mathbf{m}^3} = 2.364 \quad \delta'_{22} := \frac{\delta_{22} \cdot EJ}{\mathbf{m}^3} = 5.293 \quad \delta'_{21} := \delta'_{12}$$

$$m_0 := 100 \text{ kg} \quad m_1 = 5 m_0 \quad m_2 = 3 m_0 \quad m'_1 := 5 \quad m'_2 := 3$$

$$(m'_1 \cdot \delta'_{11} \cdot X - 1) \cdot A_1 + m'_2 \cdot \delta'_{12} \cdot X \cdot A_2 = 0$$

$$m'_1 \cdot \delta'_{21} \cdot X \cdot A_1 + (m'_2 \cdot \delta'_{22} \cdot X - 1) \cdot A_2 = 0$$

Stąd:

$$\begin{bmatrix} m'_1 \cdot \delta'_{11} \cdot X - 1 & m'_2 \cdot \delta'_{12} \cdot X \\ m'_1 \cdot \delta'_{21} \cdot X & m'_2 \cdot \delta'_{22} \cdot X - 1 \end{bmatrix} \cdot \begin{bmatrix} A_1 \\ A_2 \end{bmatrix} = 0$$

Odrzucamy rozwiązanie trywialne:

$$\begin{bmatrix} A_1 \\ A_2 \end{bmatrix} \neq 0$$

Więc:

$$\left\| \begin{bmatrix} m'_1 \cdot \delta'_{11} \cdot X - 1 & m'_2 \cdot \delta'_{12} \cdot X \\ m'_1 \cdot \delta'_{21} \cdot X & m'_2 \cdot \delta'_{22} \cdot X - 1 \end{bmatrix} \right\| = 0$$

$$(m'_1 \cdot \delta'_{11} \cdot X - 1) \cdot (m'_2 \cdot \delta'_{22} \cdot X - 1) - m'_1 \cdot \delta'_{21} \cdot X \cdot m'_2 \cdot \delta'_{12} \cdot X = 0$$

$$m'_1 \cdot m'_2 \cdot (\delta'_{11} \cdot \delta'_{22} - \delta'^2_{12}) \cdot X^2 + (-m'_1 \cdot \delta'_{11} - m'_2 \cdot \delta'_{22}) \cdot X + 1 = 0$$

$$a := m'_1 \cdot m'_2 \cdot (\delta'_{11} \cdot \delta'_{22} - \delta'^2_{12}) \quad b := -m'_1 \cdot \delta'_{11} - m'_2 \cdot \delta'_{22} \quad c := 1$$

$$\Delta := b^2 - 4 \cdot a \cdot c$$

$$X_1 := \frac{-b - \sqrt{\Delta}}{2a} = 0.044$$

$$X_2 := \frac{-b + \sqrt{\Delta}}{2a} = 0.273$$

$$\omega_1 := \sqrt{\frac{X_1 \cdot EJ}{m_0 \cdot m^3}} = 29.385 \frac{\text{rad}}{\text{s}}$$

$$\omega_2 := \sqrt{\frac{X_2 \cdot EJ}{m_0 \cdot m^3}} = 73.248 \frac{\text{rad}}{\text{s}}$$

Obliczenie amplitud drgań własnych:

$$(m'_1 \cdot \delta'_{11} \cdot X_1 - 1) \cdot A_{11} + m'_2 \cdot \delta'_{12} \cdot X_1 \cdot A_{21} = 0$$

$$m'_1 \cdot \delta'_{21} \cdot X_1 \cdot A_{11} + (m'_2 \cdot \delta'_{22} \cdot X_1 - 1) \cdot A_{21} = 0$$

$$A_{11} := 1 \quad A_{21} := \frac{-(m'_1 \cdot \delta'_{11} \cdot X_1 - 1)}{m'_2 \cdot \delta'_{12} \cdot X_1} = 1.723$$

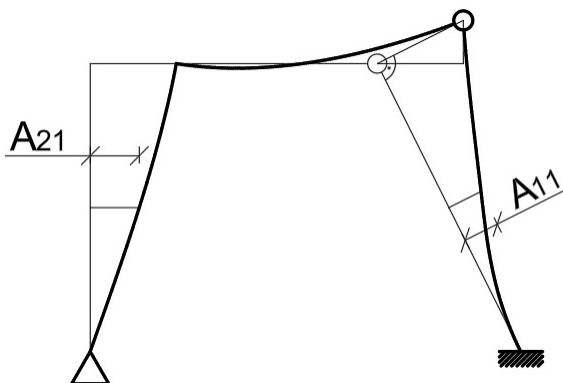
$$(m'_1 \cdot \delta'_{11} \cdot X_2 - 1) \cdot A_{12} + m'_2 \cdot \delta'_{12} \cdot X_2 \cdot A_{22} = 0$$

$$m'_1 \cdot \delta'_{21} \cdot X_2 \cdot A_{12} + (m'_2 \cdot \delta'_{22} \cdot X_2 - 1) \cdot A_{22} = 0$$

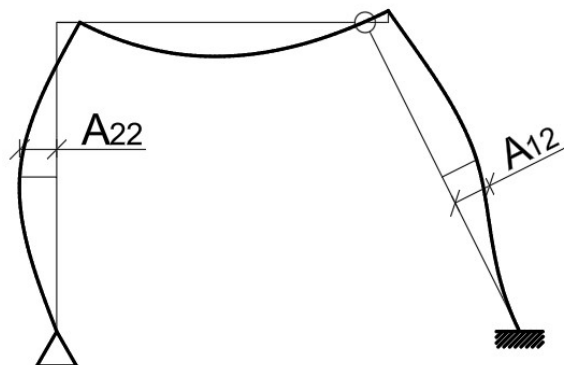
$$A_{12} := 1 \quad A_{22} := \frac{-(m'_1 \cdot \delta'_{11} \cdot X_2 - 1)}{m'_2 \cdot \delta'_{12} \cdot X_2} = -0.967$$

Formy drgań własnych

I forma drgań własnych



II forma drgań własnych



Sprawdzenie ortogonalności:

$$\text{Para 1 i 2: } m_1 \cdot A_{11} \cdot A_{12} + m_2 \cdot A_{21} \cdot A_{22} = 0 \text{ kg}$$

Sprawdzenie metodą Dunkerlaya i Reileigh'a:

$$\omega_D := \frac{1}{\sqrt{m_1 \cdot \delta_{11} + m_2 \cdot \delta_{22}}} = 27.272 \frac{\text{rad}}{\text{s}}$$

$$\delta_1 := m_1 \cdot \delta_{11} + m_2 \cdot \delta_{12}$$

$$\delta_2 := m_1 \cdot \delta_{21} + m_2 \cdot \delta_{22}$$

$$\omega_R := \frac{\sqrt{m_1 \cdot \delta_1 + m_2 \cdot \delta_2}}{\sqrt{m_1 \cdot \delta_1^2 + m_2 \cdot \delta_2^2}} = 29.535 \frac{\text{rad}}{\text{s}}$$

$$\omega_D = 27.272 \frac{\text{rad}}{\text{s}} \leq \omega_1 = 29.385 \frac{\text{rad}}{\text{s}} \leq \omega_R = 29.535 \frac{\text{rad}}{\text{s}} \quad \text{Warunek spełniony}$$

Drgania wymuszone

Sprawdzenie warunku rezonansu:

$$0.9 \omega_1 = 26.447 \frac{\text{rad}}{\text{s}} \leq \theta = 31.416 \frac{\text{rad}}{\text{s}} \leq 1.1 \omega_1 = 32.324 \frac{\text{rad}}{\text{s}}$$

$$0.9 \omega_2 = 65.923 \frac{\text{rad}}{\text{s}} \leq \theta = 31.416 \frac{\text{rad}}{\text{s}} \leq 1.1 \omega_2 = 80.573 \frac{\text{rad}}{\text{s}}$$

Warunek spełniony, rezonans zajdzie w pierwszej formie drgań własnych

Wymuszenie jest na kierunku 2, więc:

$$\delta_{1P} := \delta_{21} \cdot P_0 = 0.01324 \text{ m}$$

$$\delta_{2P} := \delta_{22} \cdot P_0 = 0.02965 \text{ m}$$

Wyznaczenie sił bezwładności:

$$\left(\delta_{11} - \frac{1}{m_1 \cdot \Theta^2} \right) \cdot B_1 + \delta_{12} \cdot B_2 + \delta_{1P} = 0$$

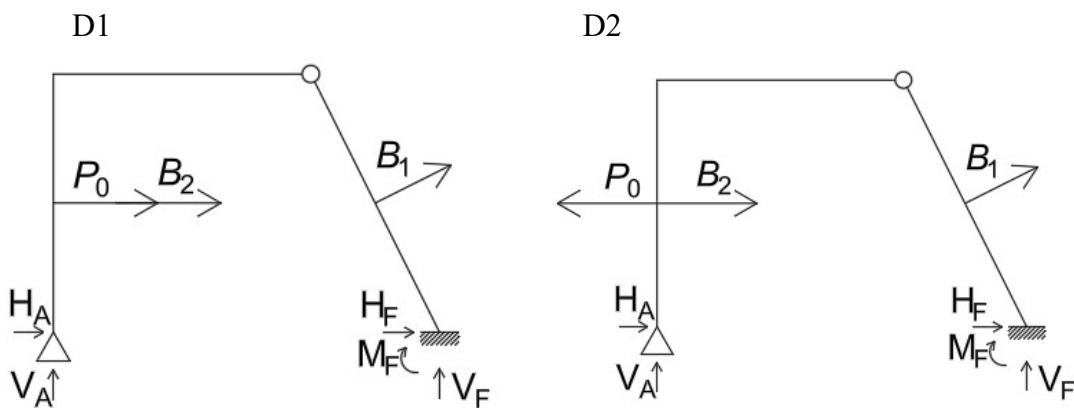
$$\delta_{11} \cdot B_1 + \left(\delta_{12} - \frac{1}{m_2 \cdot \Theta^2} \right) \cdot B_2 + \delta_{2P} = 0$$

$$\begin{bmatrix} B_1 \\ B_2 \end{bmatrix} := \begin{bmatrix} \delta_{11} - \frac{1}{m_1 \cdot \Theta^2} & \delta_{12} \\ \delta_{21} & \delta_{22} - \frac{1}{m_2 \cdot \Theta^2} \end{bmatrix}^{-1} \cdot \begin{bmatrix} -\delta_{1P} \\ -\delta_{2P} \end{bmatrix}$$

$$B_1 = -56.009 \text{ kN}$$

$$B_2 = -55.419 \text{ kN}$$

Dwa warianty obciążenia statycznego:



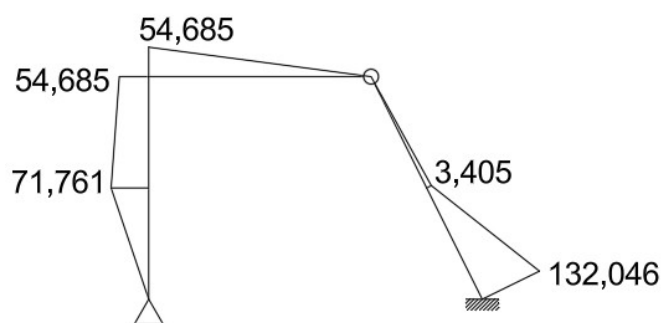
$$M_{D1} := M_{ost1} \cdot B_1 + M_{ost2} \cdot (B_2 + P_0)$$

$$M_{D2} := M_{ost1} \cdot B_1 + M_{ost2} \cdot (B_2 - P_0)$$

$$M_{D1} = \begin{bmatrix} 0 & -71.761 \\ -71.761 & -54.685 \\ -54.685 & 0 \\ 0 & -3.405 \\ -3.405 & -132.046 \end{bmatrix} \text{ kNm}$$

$$M_{D2} = \begin{bmatrix} 0 & -100.105 \\ -100.105 & -67.372 \\ -67.372 & 0 \\ 0 & -15.89 \\ -15.89 & -157.015 \end{bmatrix} \text{ kNm}$$

MD1 [kNm]



Obliczenie tnących (metoda równoważenia prętów):

$$T_{AB} := \frac{71.761 \text{ kNm}}{2 \text{ m}} = 35.881 \text{ kN}$$

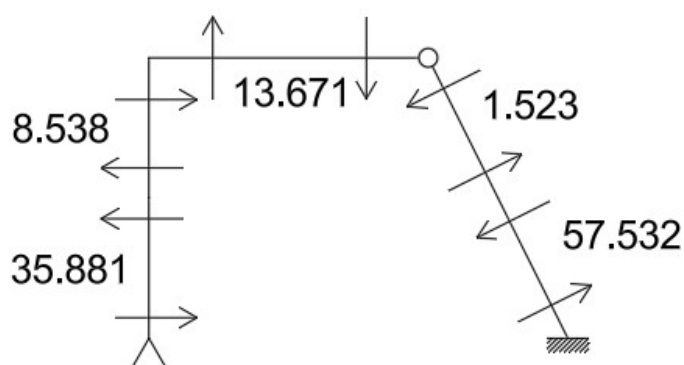
$$T_{BC} := \frac{71.761 \text{ kNm} - 54.685 \text{ kNm}}{2 \text{ m}} = 8.538 \text{ kN}$$

$$T_{CD} := \frac{54.685 \text{ kNm}}{4 \text{ m}} = 13.671 \text{ kN}$$

$$T_{DE} := \frac{3.405 \text{ kNm}}{2.236 \text{ m}} = 1.523 \text{ kN}$$

$$T_{EF} := \frac{132.046 \text{ kNm} - 3.405 \text{ kNm}}{2.236 \text{ m}} = 57.532 \text{ kN}$$

TD1 [kN]



Obliczenie Normalnych (metoda równoważenia węzłów):

Węzeł C

$$\begin{aligned} \sum X &= -8.538 \text{ kN} + N_{CD} = 0 & N_{CD} &:= 8.538 \text{ kN} \\ \sum Y &= -13.671 \text{ kN} - N_{CB} = 0 & N_{CB} &:= -13.671 \text{ kN} \end{aligned}$$

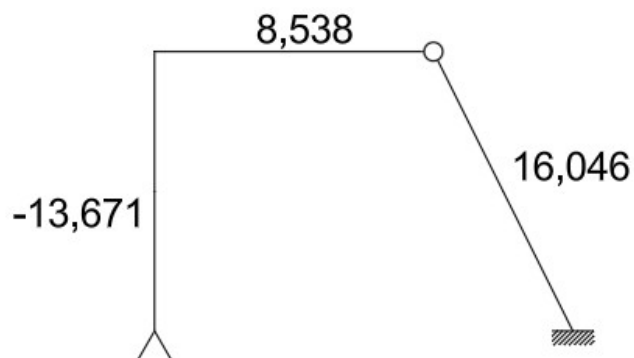
Węzeł D

$$\begin{aligned} \sum Y &= 13.671 \text{ kN} + 1.523 \text{ kN} \cdot \cos \alpha - N_{DE} \cdot \sin \alpha = 0 \\ N_{DE} &:= \frac{13.671 \text{ kN} + 1.523 \text{ kN} \cdot \cos \alpha}{\sin \alpha} = 16.046 \text{ kN} \end{aligned}$$

Sprawdzenie:

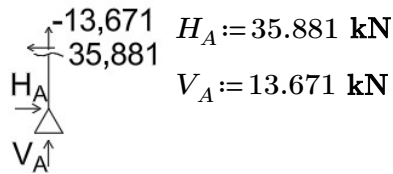
$$\sum X := -N_{CD} + N_{DE} \cdot \cos \alpha + 1.523 \text{ kN} \cdot \sin \alpha = 0 \text{ kN}$$

ND1 [kN]

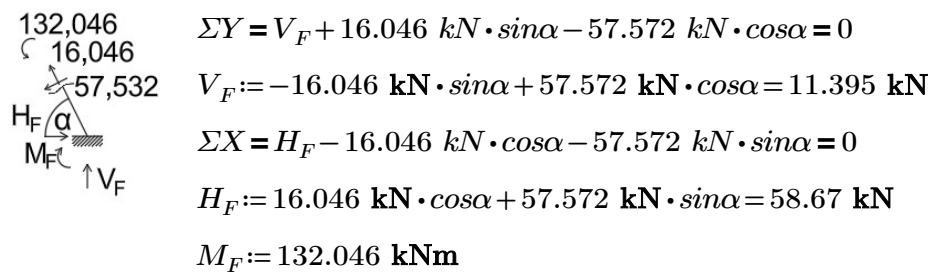


Obliczenie Reakcji (metoda równoważenia węzłów):

Węzeł A



Węzeł F



Sprawdzenie reakcji:

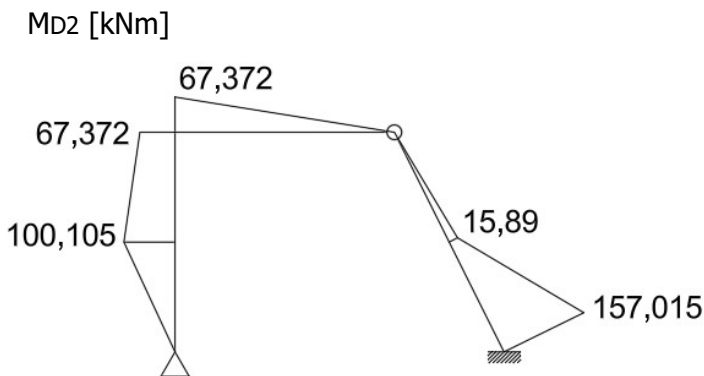
$$\Sigma X := H_A + H_F + P_0 + B_2 + B_1 \cdot \sin \alpha = 0.036 \text{ kN}$$

$$\Sigma Y := V_A + V_F + B_1 \cdot \cos \alpha = 0.018 \text{ kN}$$

$$\Sigma M_D := V_A \cdot 4 \text{ m} - H_A \cdot 4 \text{ m} - P_0 \cdot 2 \text{ m} - B_2 \cdot 2 \text{ m} - B_1 \cdot 2.236 \text{ m} - H_F \cdot 4 \text{ m} - V_F \cdot 2 \text{ m} + M_F = -0.191 \text{ kNm}$$

$$\frac{0.191 \text{ kNm}}{132.046 \text{ kNm}} \cdot 100\% = 0.145\%$$

Warunek spełniony



Obliczenie tnących (metoda równoważenia prętów):

$$T_{AB} := \frac{100.105 \text{ kNm}}{2 \text{ m}} = 50.053 \text{ kN}$$

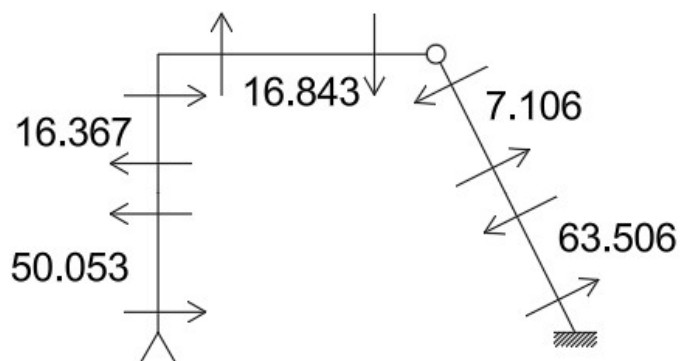
$$T_{BC} := \frac{100.105 \text{ kNm} - 67.372 \text{ kNm}}{2 \text{ m}} = 16.367 \text{ kN}$$

$$T_{CD} := \frac{67.372 \text{ kNm}}{4 \text{ m}} = 16.843 \text{ kN}$$

$$T_{DE} := \frac{15.89 \text{ kNm}}{2.236 \text{ m}} = 7.106 \text{ kN}$$

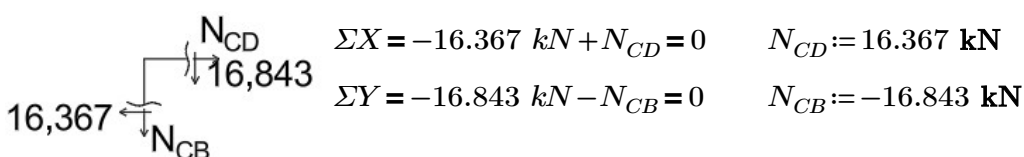
$$T_{EF} := \frac{157.015 \text{ kNm} - 15.015 \text{ kNm}}{2.236 \text{ m}} = 63.506 \text{ kN}$$

TD2 [kN]

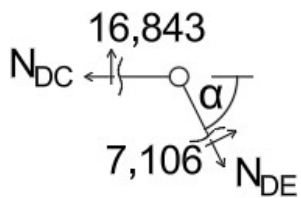


Obliczenie Normalnych (metoda równoważenia węzłów):

Węzeł C



Węzeł D



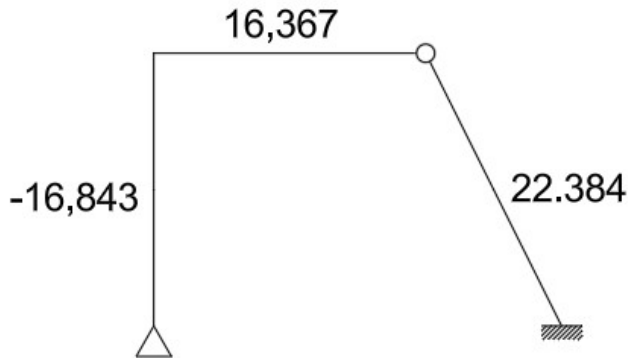
$$\Sigma Y = 16.843 \text{ kN} + 7.106 \text{ kN} \cdot \cos\alpha - N_{DE} \cdot \sin\alpha = 0$$

$$N_{DE} := \frac{16.843 \text{ kN} + 7.106 \text{ kN} \cdot \cos\alpha}{\sin\alpha} = 22.384 \text{ kN}$$

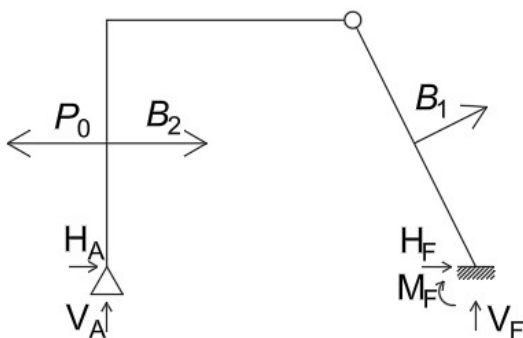
Sprawdzenie:

$$\Sigma X := -N_{CD} + N_{DE} \cdot \cos\alpha + 7.106 \text{ kN} \cdot \sin\alpha = -0.001 \text{ kN}$$

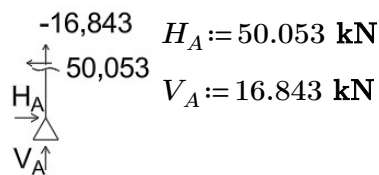
ND1 [kN]



Obliczenie Reakcji (metoda równoważenia węzłów):



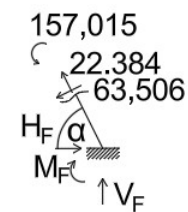
Węzeł A



$$H_A := 50.053 \text{ kN}$$

$$V_A := 16.843 \text{ kN}$$

Węzeł F



$$\Sigma Y = V_F + 22.384 \text{ kN} \cdot \sin\alpha - 63.506 \text{ kN} \cdot \cos\alpha = 0$$

$$V_F := -22.384 \text{ kN} \cdot \sin\alpha + 63.506 \text{ kN} \cdot \cos\alpha = 8.38 \text{ kN}$$

$$\Sigma X = H_F - 22.384 \text{ kN} \cdot \cos\alpha - 63.506 \text{ kN} \cdot \sin\alpha = 0$$

$$H_F := 22.384 \text{ kN} \cdot \cos\alpha + 63.506 \text{ kN} \cdot \sin\alpha = 66.812 \text{ kN}$$

$$M_F := 157.015 \text{ kNm}$$

Sprawdzenie reakcji:

$$\Sigma X := H_A + H_F - P_0 + B_2 + B_1 \cdot \sin\alpha = 0.35 \text{ kN}$$

$$\Sigma Y := V_A + V_F + B_1 \cdot \cos\alpha = 0.175 \text{ kN}$$

$$\Sigma M_D := V_A \cdot 4 \text{ m} - H_A \cdot 4 \text{ m} + P_0 \cdot 2 \text{ m} - B_2 \cdot 2 \text{ m} - B_1 \cdot 2.236 \text{ m} - H_F \cdot 4 \text{ m} - V_F \cdot 2 \text{ m} + M_F = -1.759 \text{ kNm}$$

$$\frac{1.759 \text{ kNm}}{157.015 \text{ kNm}} \cdot 100\% = 1.12\%$$

Warunek spełniony